## CE801-ESTIMATING, SPECIFICATIONS \& CONTRACTS

UNIT - I
General items of work in Building - Standard Units Principles of working out quantities for detailed and abstract estimates -Approximate method of Estimating

## UNIT - II

Rate Analysis - Working out data for various items of work over head and contigent charges.

## UNIT-III

Earthwork for roads and canals, Reinforcement bar bending and bar requirement schedules

UNIT - IV
Contracts - Types of contracts - Contract Documents - Conditions of contract, Valuation of buildings-
Standard specifications for different items of building construction

## UNIT-V

Detailed Estimation of Buildings using individual wall method.

## UNIT -VI

Detailed Estimation of Buildings using centre line method.

## FINAL EXAMINATION PATTERN:

The end examination paper should consist of SIX questions from Unit 1 to Unit 4, out of which THREE are to be answered ( $60 \%$ weight-age) \& ONE mandatory question ( $40 \%$ weight-age) from Units $5 \& 6$ is to be answered

## Unit-1

## DEFINITION OF ESTIMATING AND COSTING

Estimating is the technique of calculating or Computing the various quantities and the expected Expenditure to be incurred on a particular work or project.

In case the funds avilable are less than the estimated cost the work is done in part or by reducing it or specifications are altered, the following requirement are necessaryfor preparing an estimate.
a ) Drawings like plan, elevation and sectionsofimportantpoints.
b) Detailedspecificationsaboutworkmenship \& propertiesofmaterialsetc.
c) Standard schedule of rates of the current year.

NEED FOR ESTIMATION AND COSTING

1. Estimate give an idea of the cost of the work and hence its feasibility can be determined i..e whether the project could be taken up with in the funds available ornot.
2. Estimate gives an idea oftime required for the completionofthe work.
3. Estimate is required to invite the tenders and Quotations and to arange contract.
4. Estimate is also required to control the expenditure during the execution of work.
5. Estimate decides whether the proposed plan matches the funds available or not.

PROCEDUREOFESTIMATINGORMETHODOFESTIMATING.
Estimating involves the following operations

1. Preparing detailedEstimate.
2. Calculating the rate ofeachunit ofwork
3. Preparing abstractofestimate

DATA REQUIRED TO PREPARE AN ESTIMATE

1. Drawings i.e.plans, elevations, sections etc.
2. Specifications.
3. Rates

### 1.4.1 DRAWINGS

If the drawings are not clear and without complete dimensions the prepa- ration of estimationbecomeverydifficult. So, It is veryessential beforeprepar- ingan estimate.

## SPECIFICATIONS

a) General Specifications: This gives the nature, quality, class and work and materials in general terms to be used in various parts of wok. It helps no forma generalideaofbuilding.
b) Detailed Specifications: These gives the detailed description of the various items of work laying down the Quantities and qualities of materials, their proportions, the method of preparation workmanship and execution
of work.

## RATES:

For preparing the estimate the unit rates of each item of work are re- quired.

1. For arriving at the unit rates of each item.
2. The rates of various materials to be used in the construction.
3. The cost oftransport materials.
4. Thewagesoflabour, skilled or unskilled ofmasons, carpenters, Mazdoor, etc.,

## COMPLETE ESTIMATE:

Most of people think that the estimate of a structure includes cost of land, cost of materials and labour, But many other direct and indirect costs included and is shown below. The Complete Estimate


## LUMPSUM:

While preparing an estimate, it is not possible to workout in detail in case ofpetty items. Items other than civil engineering such items are called lumpsum itemsor simplyL.S.Items.

The following are some of L.S. Items in the estimate.

1. Water supplyand sanitaryarrangements.
2. Electricalinstallations like meter, motor, etc.,
3. Architecturalfeatures.
4. Contingenciesandunforeseenitems.

In general, certain percentage on the cost of estimation is allotted for the above L.S.Items

Even if sub estimates prepared or at the end of execution of work, the actual cost should not exceed the L.S .amounts provided in the main estimate.

During the construction of a project considerable number of skilled supervisors, work assistance, watch men etc., are employed on temporary basis. The salaries of these persons are drawn fromthe L.S. amount alloted towards the work charged establishment. that is, establishment which is charged directly to work. an L.S.amount of $11 / 2$ to $2 \%$ of the estimated cost is provided towards.

## UNITS OF MEASUREMENTS:

The units of measurements are mainly categorised for their nature, shape and size and for making payments to the contractor and also. The principle of units of measurementsnormally consists the following:
a) Single units work like doors, windows, trusses etc., are expressed in numbers.
b) Works consists linear measurements involve length like cornice, fencing, hand rail, bands of specified width etc., are expressed in runningmetres (RM)
c) Works consists areal surface measurements involve area like plastering, white washing, partitions of specified thickness etc., are expressed in square meters ( $\mathrm{m}^{2}$ )
d) Works consists cubical contents which involve volume like earth work, cement concrete, Masonry etc are expressed in Cubic metres.

| $\begin{aligned} & \text { Sl. } \\ & \text { So. } \\ & \text { No. } \end{aligned}$ | Particulas of item | Units of Measurement | Units of payment |
| :---: | :---: | :---: | :---: |
| I | Earth work: |  |  |
|  | 1. Earthwork inExcavation | cum | Per\%cum |
|  | 2. Earthworkinfillinginfoundationtrenches | cum | Per\%cum |
|  | 3. Earthworkin filling inplinth | cum | Per\%cum |
| II | Concrete: |  |  |
|  | 1. Limeconcretre in foundation | cum | percum |
|  | 2 Cement concrete inLintels | cum | percum |
|  | 3. R.C.C.inslab | cum | percum |
|  | 4. C.C. or R.C.C. Chuja, Sun- | cum | percum |
|  | 5. L.C.inroofterracing | sqm | persqm |




| VIII | Roofing <br> 1. R.C.C. and R.B.Slab roof <br> (excludingsteel) | cum | per cum |
| :---: | :--- | :---: | :---: |
| 2.L.C.roofover and inclusive <br> oftilesor brickor stoneslab <br> etc (thicknessspecified) | sqm | per sqm |  |
| 3.Centering and shuttering <br> formwork | sqm | per sqm |  |
| 4. A.C.Sheet roofing <br> Plastering, points\&finishing <br> 1. Plastering-Cement or Lime <br> Mortar (thickness and pro- <br> portionspecified) <br> 2. Pointing <br> 3. White washing, colour <br> sqm | pqm | per sqm |  |
| sqm |  |  |  |



## RULES FOR MEASUREMENT :

The rules for measurement of each itemare invaribly described in IS1200. However some of the general rules are listed below.

1. Measurement shall be made for finished itemof work and description of each item shall include materials, transport, labour, fabrication tools and plant and all types of overheads for finishing the work in required shape, sizeandspecification.
2. In booking, the order shall be in sequence of length, breadth and height or thickness.
3. Allworks shall be measured subject tothe following tolerances.
i) Linear measurement shall be measured to the nearest 0.01 m .
i) Areas shall be measured to the nearest 0.01 sq.m
iin) Cubic contents shall be worked-out to the nearest 0.01 cum
4. Same typeofworkunder different conditions and nature shall be measured separatelyunder separateitems.
5. The bill ofquantities shall fully describe the materials, proportions, workmanships and accurately represent the work to be executed.
6. In case of masonary(stone or brick) or structuralconcrete, the categories shall be measured separatelyand the heights shall be described:
a) fromfoundationto plinth level
b) fromplinthlevelto First floor level
from Fist floor to Second floor level and so on.

## METHODS OFTAKINGOUT QUANTITIES:

The quantities like earthwork, foundationconcrete, brickwork in plinth and super structure etc., canbe workout by anyof following two methods:
a) Long wall-short wall method
b) Centre linemethod.
c) Partlycentre line and short wall method.

## a) Longwall-short wallmethod:

In this method, thewallalongthe length ofroomis consideredto be long wallwhile the wall perpendicular to long wall is said to be short wall. To get the
lengthoflongwallor short wall, calculate first thecentreline lengthsofindividual walls. Then the length of long wall, (out to out) may be calculated after adding half breadth at each end to its centre line length. Thus the length of short wall measuredinto in and may be found by deductinghalf breadthfrom its centre line length at each end. The lengthof long wallusuallydecreases fromearthworkto brick work in super structure while the short wall increases. These lengths are multiplied by breadth and depthto get quantities.

## b) Centre linemethod:

This method is suitable for walls of similar cross sections. Herethe total centre line length is multiplied by breadthand depth of respective itemto get the total quantity at a time. When cross walls or partitions or verandah walls join with mainall, the centre line length gets reduced by half of breadth for each junction. such junctionor joints are studied caefullywhile calculating totalcentre line length.The estimates prepared by this method are most accurate and quick.
c) Partly centre line and partly cross wall method:

This method is adoptedwhenexternal(i.e., alroundthebuilding) wall is ofonethicknessandtheinternalwallshaving different thicknesses. In suchcases, centre line method is applied to externalwallsand longwall-shortwallmethodis usedto internalwalls. Thismethodsuitsfordifferentthicknesseswallsanddiffeent level of foundations. Because of this reason, all Engineering departments are practicing thismethod.

Typyes of estimates:

## DETAILED ESTIMATE:

The preparationof detailed estimate consists of working out quantities of various items ofworkand thendetermine the cost of each item. This is prepared in twostages.

## i) Details of measurements and calculation of quantities:

The complete work is divided into various items of work such as earth workconcreting, brick work, R.C.C. Plastering etc., The details of measurements are taken from drawings and entered in respective columns of prescribed proforma. the quantities arecalculated by multiplying the values that are in numbers columnto Depth column as shownbelow:

## Details of measurements form

| S.No | Description <br> of Item | No | Length <br> (L) <br> m | Breadth <br> (B) <br> m | Depth/ <br> Height <br> (D/H)m | Quantity | Explanatory <br> Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |

## i) Abstract of Estimated Cost :

The cost of each item of work is worked out from the quantities that already computed in the detals measurement form at workable rate. But the total cost is worked out in theprescribedform is known as abstract of estimated form. 4\%of estimated Cost is allowedfor Petty Supervision, contingencies and Unforeseenitems

| ItemNo. | Description/ <br> Particulars | Quantity | Unit | Rate | Per <br> (Unit) | Amount |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

The detailed estimate should accompained with
i) Report
i) Specification
ii) Drawings(plans, elevation, sections)
iv) Designchartsandcalculations
v) Standard schedule of rates.
3.1.1.Factors to be consisdered While Preparing Detailed Estimate:
i) Quantity and transportation of materials: For bigger project, the requirement of materials is more. suchbulk volume of mateials will be purchased and transported definitely at cheaper rate.
i) Location of site: The site of work is selected, such that it should reduce damageor in transit during loading, unloading, stockingofmateirals.

Li*) Local labour charges: The skill, suitability and wages of local laboures
are consideed while preparing the detailed estimate.

## DATA:

The process of working out the cost or rate per unit of each item is called as Data. In preparation of Data, the rates of materials and labour are obtained from current standard scheduled of rates and while the quantities of materials and labour required for one unit of item are taken from Standard Data Book (S.D.B)

## Fixing of Rate per Unit of an Item:

The rate per unit of an item includes the following:

1) Quantity of materials \& cost: The requirement of mateials are taken strictly in accordance with standard data book(S.D.B). The cost of these includes first cost, freight, insurance and transportationcharges.
ii) Cost of labour: The exact number of labourers required for unit of work and the multiplied bythe wages/ dayto get of labour for unit itemwork.
i) Cost of equipment $(\boldsymbol{T} \& \boldsymbol{P})$ : Some works need special type of equipment, tools and plant. In such case, an amount of 1 to $2 \%$ of estimated cost isprovided.
Overhead charges: To meet expenses of office rent, depreciation of equipment salaries ofstaff postage, lighting an amount of $4 \%$ ofestimate cost isallocated

## Unit-2

Definition : In order to determine the rate of a particular item, the factors affecting the rate of that item are studied carefully and then finally a rate is decided for that item. This process of determining the rates of an item is termed as analysis ofrates or rate analysis.
The rates of particular item of work depends on the following.

1. Specificationsofworksand materialabouttheir quality, proportionandconstructional operationmethod.
2. Quantityof materials and their costs.
3. Cost of labours and their wages.
4. Location of site of work and the distances from source and conveyance charges.
5. Overhead and establishment charges
6. Profit

## Cost of materials at source and at site of construction.

The costs of materials are taken as delivered at site inclusive of the transport localtaxes and other charges.
Purpose of Analysis of rates:

1. To workout the actual cost of per unit of the items.
2. To workout the economical use of materials and processes in completing theparticulars item.
3. To workout the cost of extra items which are not provided in the contract bond, but are to be done as per the directions of the department.
4. To revise the schedule of rates due to increase in the cost of material and labour or due to change in technique.

## Cost of labour -types of labour, standard schedule of rates

The labour can be classified in to

1) Skilled 1stclass
2) Skilled IInd Class
3) unskilled

The labour charges can be obtained fromthe standard schedule of rates $30 \%$ of the skilled labour provided in the data may be taken as Ist class, remaining $70 \%$ as II class. The rates of materials for Government works are fixed by the superintendent Engineer for his circle every year and approved by the Board of Chief Engineers. These rates are incorporated in the standard schedule of rates.
Lead statement: The distance between the source of availability of material and construction site is known as "Lead " and is expected in Km . The cost of convenayce of material depends on lead.

This statement will give the total cost of materials per unit item. It includes first cost, convenayce loading, unloading stacking, chargesetc.

The rate shown in the lead statement are for mettalled road and include loading and staking charges. The environment lead on the metalled roads are arrived by multiplying by a factor
a) for metaltracks - lead x 1.0
b) For cartze tracks - Lead x1.1
c) For Sandy tracks - lead x1.4

Note: For $1 \mathrm{~m}^{3}$ wet concrete $=1.52 \mathrm{~m}^{3}$ dry concrete approximately
SP.Wt of concrete $=1440 \mathrm{~kg} / \mathrm{m}^{3}$ (or) $1.44 \mathrm{t} / \mathrm{m}^{3}$
1 bag of cement $=50 \mathrm{Kg}$
Example 1:- Calculate the Quantity of material for the following items.
a) R.C.C. $(1: 2: 4)$ for $20 \mathrm{~m}^{3}$ of work
b) R.C.C. $(1: 3: 6)$ for $15 \mathrm{~m}^{3}$ of work
a) Quantityof cement required $=\frac{1}{(1+2+4)} \times 1.52 \times 20=4.14 \mathrm{~m}^{3} \times \frac{1440}{50}$

$$
=119.26 \mathrm{bags}
$$

2
Quantity of Sand required $=\overline{(1+2+4)} \times 1.52 \times 20=8.28 \mathrm{~m}^{3}$
4
Quantity of cource aggreate $={ }_{7} * 1.52 \times 20=16.56 \mathrm{~m}^{3}$
b) Quantity of cement required $=\frac{1}{10} \underline{\times 1} .52 \times 1.5=2.28 \mathrm{~m}^{3} \times \begin{aligned} & 1440 \\ & 50 \\ & =89.88\end{aligned}$

3
Quantityof sand required $=10 * 1.52 \times 15=6.84 \mathrm{~m}^{3}$
6
QuantityofCA required $={ }_{10} \times 1.52 \times 15=13.68 \mathrm{~m}^{3}$

Example 5:- Prepare the lead statement for the following materials

| S.No. | Material | Rate at Source | Lead in KM |  |  | Conveyance Charge per km |
| :--- | :--- | :--- | :---: | :---: | :---: | :---: |
|  |  |  | MT | CT | ST |  |
| 1. | 40 mm HBG Metal | Rs.120/m | --- | 5 | 7 | Rs. $5.00 / \mathrm{m}^{3}$ |
| 2. | River Sand | Rs.15 $/ \mathrm{m}^{3}$ | 3 | 2 | 6 | Rs.3.50/m² |
| 3. | Cement | Rs. 135/bags | 2 | --- | 4 | Rs. 4.00 per $4 \mathrm{~km} / \mathrm{bag}$ |


| S.No | Mateial | Rate of Source | Lead in KM |  |  | Equalant lead in km | Conveyance Charge | Total conveyance Charge | Total cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | MT | CT | ST |  |  |  |  |
| 表1. | 40 mm HBG Metal | Rs. $120 / \mathrm{m}^{3}$ | -- | 5 | 7 | $5 \times 1.1+7 \times 1.4=15.3$ | 5.00/m3 | $15.3 \times 5=76.5$ | $120+76.5=196.5 / \mathrm{m}^{3}$ |
| 事2. | River Sand | Rs.15/m ${ }^{3}$ | 3 | 2 | 6 | $\begin{aligned} & 3 \times 1+2 \times 1.1+6 \times 1.4 \\ & =13.6 \end{aligned}$ | 3.50/m3 | $13.6 \times 3.5=47.6$ | $15+47.6=62.6 / \mathrm{m}^{3}$ |
| 3. | Cement | Rs. 135/bags | 2 | --- | 4 | $2 \times 1+4 \times 1.4=7.6$ | 4.00per4km/bae | $\begin{aligned} & 7.6 \\ & 4.0 \times 4.0=7.6 \end{aligned}$ | $135+7.6=142.6 / \mathrm{bag}$ |

Cost of cement at site $=142.6 / \mathrm{bag}$
1 bag of cement $=50 \mathrm{~kg}$
sp.wt of cement $=1440 \mathrm{~kg} / \mathrm{m} 3=1.44 \mathrm{t} / \mathrm{m} 3$
1440
Cost of Cement $=142.6 \mathrm{x} 50 \quad=4106.88 / \mathrm{m}^{3}$

Example 6:- Prepare the lead statement for the following materials

| S.No. | Material | Rate of Source | Lead in KM |  |  | Conveyance Charge per km | Seinarage Charges | CessCharges |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | ST | CT | MT |  |  |  |
| 1. | Cement | Rs.2100/10KN(tonn) | 5 | 2 | 3 | Rs.1.5/m ${ }^{3}$ | --- | --- |
| 2. | Bricks | Rs.850/100nos | 5 | -- | 3 | Rs.30/1000Nos/Km | 35 | 13 |
| 3. | Sand | Rs. $15 / \mathrm{m}^{3}$ | 4 | 2 | 5 | Rs. 9.00 / km/cum | 30 | 12 |
| 4. | 40mm HBGMetal | Rs. $250 / \mathrm{m}^{3}$ | 3 | 2 | 2 | Rs. $6.50 / \mathrm{Km} / \mathrm{m}^{3}$ | 35 | 15 |


| S.No | Material | Rate of Source | L : ad in KM |  |  | Equalant lead in km | Conveyance Charge Rs. | Total conveyance Charge Rs. | Seinerage Charge Rs. | $\begin{gathered} \hline \text { Cess } \\ \text { Charg } \\ \text { Rs. } \end{gathered}$ | Total cost Rs. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | ST | CT | MT |  |  |  |  |  |  |
| 1. | Cement | Rs.2100/10KN | 5 | 2 | 3 | $5 \times 1.4+2 \times 1.1+3 \times 1=11.2$ | 1.50 | 16.80 | -- | -- | 2116.8/10KN |
| 2. | Bricks | Rs.850/1000nos | 5 | -- | 3 | $5 \times 1.4+3 \times 1=10$ | 30 | 300.00 | 35 | 13 | 1198/1000nos |
| 3. | Sand | Rs. $15 \mathrm{~m}^{3}$ | 1 | 2 | 2 | $1 \times 1.4+2 \times 1.1+2 \times 1=5.6$ | $9.00 / \mathrm{m}^{3}$ | 50.40 | 30 | 12 | 107.4/m ${ }^{3}$ |
| 4. | 40 mmHBG <br> Metal | Rs. $250 / \mathrm{m}^{3}$ | 3 | 2 | 2 | $3 \times 1.4+2 \times 1.1+2 \times 1=8.4$ | $6.5 / \mathrm{m}^{3}$ | 54.6 | 35 | 15 | 354.6/m ${ }^{3}$ |

Preparation of Unit rates for finished items of words
a) Cement Concrete in foundation (1:5:10)

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. | 40 mm HBG Metal | 0.92 | Cum | 547.75 | Cum | 503.93 |
| 2. | Sand | 0.46 | cum | 284.80 | Cum | 131.00 |
| 3. | Cement | 0.092 | Cum | 2700.00 | MT | 357.70 |
| 4. | Mason Ist Class | 0.06 | No | 150.00 | Nos | 9.00 |
| 5. | Mason 2nd Class | 0.14 | No | 131.00 | Nos | 18.34 |
| 6. | Man mazdoor | 1.80 | No | 101.00 | Nos | 181.80 |
| 7. | Women Mazdoor | 1.40 | No | 101.00 | Nos | 141.40 |
| 8. | Add Extra 15\%onM.L |  |  |  |  | 52.58 |
|  |  |  |  |  |  | 1395.75 |
| 9 | Add T.O.T. @4\% |  |  |  |  | 55.83 |
| 10 | Sundries |  |  |  |  | 0.42 |
|  |  |  |  | Total R | s. | 1452.00 |

## b). Cement Concrete in foundation (1:4:8)

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. | 40mm HBG Metal | 0.92 | Cum | 547.75 | Cum | 503.93 |
| 2. | Sand | 0.46 | Cum | 284.80 | Cum | 131.00 |
| 3. | Cement | 0.115 | Cum | 2700.00 | MT | 447.12 |
| 4. | Mason Ist Class | 0.06 | No | 150.00 | Nos | 9.00 |
| 5. | Mason 2nd Class | 0.14 | No | 131.00 | Nos | 18.34 |
| 6. | Man mazdoor | 1.80 | No | 101.00 | Nos | 181.80 |
| 7. | Women Mazdoor | 1.40 | No | 101.00 | Nos | 141.40 |
| 8. | Add Extra 15\%onM.L |  |  |  |  | 52.58 |
|  |  |  |  |  |  | 1485.17 |
| 9 | Add T.O.T. @4\% |  |  |  |  | 59.40 |
| 10 | Sundries |  |  |  |  | 0.43 |
| Total Rs. |  |  |  |  |  | 1545.00 |

## 2) R.C.C.Works

V.R.C.C.(1:2:4) Nominal mix using 20 mm Normal size hard broken granitemetalapprovedquarrywith necessaryreinforcement includingcasting, curingcost \& conveyance of all materials

2 a) P.C.C.(1:2:4)

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |  |  |
| :---: | :--- | :--- | :--- | ---: | :--- | ---: | :---: | :---: |
| 1. | 20mm HBG Metal | 0.92 | Cum | 797.75 | Cum | 733.93 |  |  |
| 2. | Sand | 0.46 | cum | 284.80 | Cum | 131.00 |  |  |
| 3. | Cement | 0.23 | Cum | 2700.00 | MT | 894.24 |  |  |
| 4. | Mason Ist Class | 0.2 | No | 180.00 | Nos | 30.00 |  |  |
| 5. | Man mazdoor | 1.8 | No | 131.00 | Nos | 235.80. |  |  |
| 6. | Women Mazdoor | 1.4 | No | 101.00 | Nos | 141.40 |  |  |
| 7. | Vibrating charges | 1.0 | Cum | 101.00 | Nos | 101.00 |  |  |
| 8. | Machinymixingconcrete 1.0 | Cum | 28.80 | cum | 28.80 |  |  |  |
| 9 | Add Extra 15\%onM.L | Total Rs. |  |  |  |  |  | 2372.40 |
|  |  |  |  |  |  |  |  |  |

b) For steelreinforcement

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :--- | :--- | :--- | ---: | :---: | ---: |
| 1. | cost of steel | 1.00 | MT | 27500 | MT | 27500.00 |
| 2. | Fabrication charges | 1.00 | MT | 5.00 | Kg | 5000.00 |
|  |  |  |  |  |  | 750.00 |


| 3. | Add 15\% on M.L. |  |  |  | 33250.00 <br> 1330.00 <br> 4. <br> 5. <br> 5. <br>  <br> Add T.O.T. @4\% <br> Sundries |
| :--- | :--- | :--- | :--- | :--- | ---: |
|  |  |  |  |  | 0.00 |

c).V.R.C.C (1:2:4) for bed blocks, column footings including form work centering charges

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :--- | :---: | :--- | :---: | :---: | ---: |
| 1. | V.P.C.C $(1: 2: 4)$ | 1.00 | Cum | 2372.40 | Cum | 2372.40 |
| 2. | Centering Charges | 1.00 | Cum | 430.00 | Cum | 430.00 |
| 3. | Steel @0.5\% $=0.5 /$ |  |  |  |  |  |
|  | $100=0.005 \mathrm{~m}^{3}$ |  |  |  |  |  |
|  | $(0.005 \times 7.85 \mathrm{t} / \mathrm{m} 3=$ | 0.04 | MT | 34580.00 | MT | $\frac{1383.20}{\mathbf{4 1 8 5 . 6 0}}$ |
| 4 | 0.04 t |  |  |  |  | 167.40 <br> 4. Add T.O.T. @4\% |
|  | Sundries |  |  |  | 0.00 |  |

## d). V.R.C.C(1:2:4) for columns rectangular beams,

 pedastals including form work at centering charges| S.No. | Descrtiption of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. | V.P.C.C. (1:2:4) | 1.00 | Cum | 2372.40 | Cum | 2372.40 |
| 2. | Centering Charges | 1.00 | Cum | 675.00 | Cum | 675.00 |
| 3. | Steel for columns, beam | s 0.117 | MT | 34580.00 | MT | $\underline{4072.00}$ |
|  | @ $1.5 \%=1.5 /$ |  |  |  |  | 7119.40 |
|  | $100 \times 7.85=0.117 \mathrm{t}$ |  |  |  |  |  |
| 4. | Add T.O.T. @4\% |  |  |  |  | 284.77 |
| 5. | Sundries |  |  |  |  | 0.83 |
|  |  |  |  | Total Rs. |  | 7405.00 |

e).V.R.C.C(1:2:4) for slabs, lintels including form work at centering charges upto 100 mm , thick

| S.No. | Descrtiption of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :--- | :---: | :--- | :--- | :--- | ---: |
| 1. | V.P.C.C $(1: 2: 4)$ | 1.00 | Cum | 2372.40 | Cum | 2372.40 |
| 2. | Centering Charges | 10.00 | Cum | 710.00 | Cum | 710.00 |
| 3. | Steel for slabs | 0.0785 | MT | 34580.00 | MT | $\underline{2714.53}$ |
|  | $@ 1 \%=1 / 100 \times 7.85=$ |  |  |  |  | $\mathbf{5 7 9 6 . 6 3}$ |
|  | 0.0785 t |  |  |  |  |  |


| Add T.O.T. @4\% |
| :--- | :--- | :--- | :--- |
| Sundries |\(\left|\begin{array}{ll}231.87 <br>

1.20\end{array}\right|\)
3. Pointing to R.R.Masonary in CM(1:4) mix using cost \& conveyance of Cement, sand and all materials from approved sources to site and labour charges for point neatly etc.

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1. | Cost of CM(1:4) | 0.09 | Cum |  | Mt | 87.48 |
|  | Cement = |  |  |  |  |  |
|  | $\times 1.44 \times 0.09$ | 0.032 | t | 2700.00 |  |  |
| 2. | $\text { Sand }=\frac{1}{4} \times 0.09$ | 0.09 | Cum | 284.80 | Cum | 25.63 |
| 3. | Mining Charges | 1.0 | Cum | 32.50 | Cum | 32.50 |
| 4. | mason Ist Class | 0.48 | Nos. | 150.00 | Nos | 72.00 |
| 5. | 2nd Class | 1.12 | Nos | 131.00 | Nos | 146.72 |
| 6. | Man mazdoor | 0.50 | Nos | 101.00 | Nos | 55.00 |
| 7. | Women Mazdoor | 1.10 | Nos | 101.00 | Nos | 111.10 |
| 8. | Add 15\% on ML |  |  |  |  | 57.72 |
|  |  |  |  |  |  | 588.15 |
| 9. | Add TOT @ 4\% |  |  |  |  | 23.53 |
| 10. | Sundries |  |  |  |  | 0.32 |
|  |  |  | Total R |  |  | 612.00 |

Cement concrete flooring ( $1: 2: 4$ ) using 12 mm HBG machine crushed chips from approved quarry to site of work including curing cost and conveyance of all materials completed.

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :--- | :--- | :--- | :--- | :--- | :--- | ---: |


| $\begin{aligned} & 1 . \\ & 2 . \\ & 3 . \\ & 4 . \end{aligned}$ | 12 mm HBG metal | 0.92 | Cum | 680.25 | cum <br> cum <br> mt <br> nos <br> nos <br> nos <br> nos | 625.8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | crushed chips |  |  |  |  |  |
|  | Sand | $\begin{array}{\|l\|} 0.46 \\ 0.23 \\ \text { (or) } 0.331 \\ 0.06 \\ 0.14 \\ 1.80 \\ 1.40 \end{array}$ | cum <br> cum <br> MT <br> Nos <br> nos <br> nos <br> nos | $\begin{array}{r} 284.80 \\ 2700 \\ \\ 150.00 \\ 131.00 \\ 101.00 \\ 101.00 \end{array}$ |  | 131.0 |
|  | Cement |  |  |  |  | 894.2 |
|  | $\left(0.23 \mathrm{~m}^{3} \mathrm{x} 1.44=0.33 \mathrm{t}\right.$ |  |  |  |  |  |
| 5. | Mason ISt class |  |  |  |  | 9.0 |
| 6. | 2nd Class |  |  |  |  | 18.3 |
| 7. | Man mazdoor |  |  |  |  | 181.8 |
| 8. | Women Mazdoor |  |  |  |  | 141.4 |
| 9. | Add 15\% Extraon ML |  |  |  |  | 52.5 |
|  |  |  |  |  |  | 2054.1 |
| 10 | Add TOT @ 4\% |  |  |  |  | 82.1 |
| 11. | Sundries |  |  |  |  | 0.6 |
|  |  |  |  | TotalR |  | 2137.0 |

5 a) Supply and fixing teak wood fully panneled with $10 \times 4 \mathrm{~cm}$ styles, and 10 x 4 cmrails and 3.5 CM TH panels with teak wood framof 6.25 x 10 cm size including cost of hold fasts, but hinges and labour charges for fixing door in positionandfixingfurniture etc., complete for one door of size $1.100 \times 2.00$ of area 2.2 sqm


## Requirements :

i) Verticals $=2 \times 2.0 \times 0.10 \times 0.0625=0.0250$
ii) Horizontals $=1 \times 1.10 \times 0.10 \times 0.0625=0.0068$
iii) Styles $=4 \times 1.937 \times 0.10 \times 0.04=0.0300$
iv) Rails $\quad=2 \times 5 \times 0.5075 \times 0.10 \times 0.04=0.0020$
v) Planks $=2 \times 4 \times 0.364 \times 0.3475 \times .035=\frac{0.0354}{\mathbf{0 . 0 0 9 0 m}^{3}}$

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :--- | :--- | :--- | :---: | :---: | ---: |
| 1. | Wood Cost | 0.009 | Cum | 25000 | cum | 2470.00 |
| 2. | Butt Hinges | 6 | Nos | 20 | each | 120.00 |
| 3. | Z-hold fasts | 6 | Nos | 10 | each | 60.00 |
| 4. | Cost of labour | 2.2 | sqm | 800 | sqm | 1760.00 |
|  |  |  |  |  | Total | $\mathbf{4 4 1 0 . 0 0}$ |

5 b) Supply and fixing teak wood fully panneled with $10 x 4 \mathrm{~cm}$ styles, and 10 x 4 cm rails and 3.5 CM TH panels with teak wood fram of 6.25 x 10 cm size including cost of hold fasts, but hinges and labour charges for fixing window in position and fixing furniture etc., complete for one window of size $1.0 \times 1.2$ of area 1.2 sqm .


## Requirements :

i) Verticals $=3 \times 1.2 \times 0.10 \times 0.0625=0.0225$
ii) Horizontals $=3 \times 1.00 \times 0.10 \times 0.0625=0.0188$
iii) Styles $=4 \times 2 \times 0.10 \times 0.04=0.0160$
iv) Rails $\quad=4 \times 2 \times 0.4062 \times 0.10 \times 0.04=0.0012$
v) Planks $=4 \times 0.3102 \times 0.2102 \times 0.03=\frac{0.0070}{\mathbf{0 . 0 0 7 6 \mathbf { m } ^ { 3 }}}$

| S.No. | Description of Item | Quantity | Unit | Rate | Per | Amount |
| :---: | :--- | :--- | :--- | :--- | ---: | ---: |
| 1. | wood Cost | 0.0076 | Cum | 25000 | cum | 1900.00 |
| 2. | Butt Hinges | 6 | Nos | 20 | each | 120.00 |
| 3. | Z-hold fasts | 4 | Nos | 10 | each | 40.00 |
| 4. | Cost of labour | 1.2 | sqm | 1000 | sqm | 1200.00 |
|  |  |  |  |  | Total | $\mathbf{3 2 6 0 . 0 0}$ |

Cost of door per $1 \mathrm{~m}^{2}=3260 / 1.2=2716.67$ say Rs.2720/-

1) Prepare the Bar bending schedule for the beam shown below.

2) Prepare the Bar bending schedule of a simply supported R.C.C. Lintels from the following specification:
Size of lintel 300mm widex 200mm depth.Main bars in tension zone of Fe 250 (grade I) 3 bars of 16 mm dia., one bar is cranked through $45^{\circ}$ at 170 mm from each end
2 No. anchor bars at top 8 mm dia.
Two legged stirrups@150mm c/c of 6mm dia. through out.
Clear span of the lintel is 1150 mm .
Bearing on either side is 150 mm .

# EARTH WORK CALCULATIONS 

## Introduction:-

Generally all the Civil Engineering projects like roads, railways, earth dams, canal bunds, buildings etc. involves the earth work.This earth work may be either earth excavation or earth filling or Some times both will get according to the desired shape and level. Basically the volume of earthwork is computed from length, breadth, and depth of excavation or filling.

In this chapter the various methods of calculating the earth work quantities shall bediscussed.

## Lead and Lift:

## Lead:

It is the average horizontal distance between the centre of excavation to the centre of deposition. The unit of lead is 50 m .

## Lift:

It is the average height through which the earth has to be lifted from source to the place of spreading or heaping. The unit of lift is 2.00 m for first lift and one extra lift for every 1.0 m . for example when earth is to be lifted for 4.5 m , Four lifts are to be paid to the contractor.
$\left.\begin{array}{rl}\text { i.e. Upto2.0- } & 1 \text { lift } \\ 1.0- & 1 \text { Lift } \\ 1.0- & 1 \text { lift } \\ 0.5- & 1 \text { lift }\end{array}\right\}$ Total 04lifts

## Calculation of earth work for Roads:

case 1) volume of earth work in banking or in cutting having "no longitudi- nal slope".

## Earth work Calculations



Voluturiesor Enerobsectional areax Mentiff ${ }^{\text {etion of a Canal in Cuting }}$

$$
\begin{aligned}
\mathrm{V} & =(\mathrm{bd}+2 \mathrm{x} 1 / 2 \mathrm{x} n d x d) \mathrm{L} \\
\mathrm{~V} & =\left(\mathrm{bd}+\mathrm{nd}^{2}\right) \mathrm{L}
\end{aligned}
$$

## Case 2:

hen the ground is in longitudinal slope or the formationhas uniform gradi- ent for a length the earth work may be calculated by the following methods.

## 1. By Mid Section or Mid ordinate method.



Where $\mathrm{d}_{1}, \mathrm{~d}_{2}=$ depth of banksat two ends

Mid ordinate (or)Average depth $\left(\mathrm{d}_{\mathrm{m}}\right)=\frac{\mathrm{d}_{1}+\mathrm{d}_{2}}{2}$
Area of mid section $(\mathrm{Am})=\left(\mathrm{bd}_{\mathrm{m}}+\mathrm{nd}_{\mathrm{m}}^{2}\right)$
volume of earth work $(v)=A_{m} \times L=\left(\operatorname{bd}_{m}+\mathrm{nd}_{\mathrm{m}}^{2}\right) \times \mathrm{L}$
ii) Trepezoidal formula: (for two sections)

In this method also called mean sectional area method
Let $\mathrm{A}_{1} \& \mathrm{~A}_{2}$ be two areas at two ends.
$\mathrm{A}=\left(\mathrm{bd}+\mathrm{nd}_{1}^{2}\right), \quad \mathrm{A}=\left(\mathrm{bd}+\mathrm{nd}^{2}\right){ }_{2}$
$\mathrm{A}_{\mathrm{m}}=\frac{\mathrm{A}_{1}+\mathrm{A}_{2}}{2}$
Volume of earth work (v) $=\mathrm{Am} \times \mathrm{L}$
iii) Trepezoidal formula for a series of $\mathrm{c} / \mathrm{s}$ areas at equal intervals.

Let $A_{1}, A_{2}, A_{3} \quad A_{n}$ are the cross sectional areas along L.S of Road 'L" is the distance between two cross sections
The volume of earth work

$$
\begin{aligned}
& \mathrm{V}=\left[\left(\frac{\mathrm{L}_{1}+\mathrm{A}_{\mathrm{n}}}{2}\right)^{[ }+\left(\begin{array}{cc}
\mathrm{A}_{2}+\mathrm{A}_{3}+\ldots . .+\mathrm{A}^{\mathrm{n}-1}
\end{array}\right)\right. \\
& =\frac{\mathrm{L}}{2}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}\right)+2\left(\mathrm{~A}_{2}+\mathrm{A}_{3}+\square+\mathrm{A}_{\mathrm{n}-1}\right)\right] \\
& \left.=\frac{\text { length }}{2}[\text { (sum of first and last areas })+2 \text { (remaing Areas) }\right]
\end{aligned}
$$

iv) Prismoidal formula for a series of cross sectional areas at equal intervals.

Note : This method is adopted when there is odd number of cross sections.
Volume of earth work

$$
\begin{aligned}
\mathrm{V} & =\frac{\mathrm{L}}{3}\left[(\underset{1}{(\mathrm{~A}}+\underset{\mathrm{n}}{\mathrm{~A}})+4\left(\mathrm{~A}_{2}+\underset{4}{\mathrm{~A}}+\mathrm{A}_{6}+\square+\mathrm{A}_{\mathrm{n}-1}\right)+\underset{3}{\left.2\left(\mathrm{~A}_{3}+\mathrm{A}_{5}+\ldots . .+\mathrm{A}_{\mathrm{n}-2}\right)\right]}\right. \\
& \left.=\frac{\text { length }}{3}(\text { Sum of first and last areas })+4(\text { even areas })+2(\text { odd Areas })\right]
\end{aligned}
$$

## Earth work Calculations

Example 7.1 : Find the volume of earth work in embankment of length 12 m .
Top width is 5.5 m and depth is 2.5 m the side slopes ara $1 \frac{1}{2}$ : 1
Sol : Top width $\mathrm{b}=5.5 \mathrm{~m}$
Depth $\mathrm{d}=2.5 \mathrm{~m}$
side slopes $=1 \frac{1}{2}$ : 1 i.e. $n=1.5$
length $\mathrm{L}=12 \mathrm{~m}$


Volume of earth work $\mathrm{V}=\left(\mathrm{bd}+\mathrm{nd}^{2}\right) \mathrm{L}$

$$
\begin{aligned}
& =\left(5.5 \times 2.5+1.5 \times 2.5^{2}\right) 12 \\
& =77.5 \mathrm{~m}^{3}
\end{aligned}
$$

Example 7.2 : The depths at two ends of an embankment of road of length 70 m are 2 m and 2.5 m . The formation width and side slopes are 8 m and $2: 1$ respectively. Estimate the Quantity of earth work by
a) Mid Sectional Area (ii)Mean sectional Area method.

Sol: a) $b=8 m, d 1=2 m, d 2=2.5 m, l=70 m, n=2$
Mean depth $\mathrm{d}_{\mathrm{m}}=\frac{\mathrm{d}_{1}+\mathrm{d}_{2}}{2}=\frac{2+2.5}{2}=2.25 \mathrm{~m}$
Mid sectional Area $=\mathrm{Am}=\mathrm{bdm}+\mathrm{ndm}^{2}=\left(8 \times 2.25+2 \times 2.25^{2}\right) 2=28.125 \mathrm{~m}^{2}$
Volume of earth work $(\mathrm{V})=\mathrm{AmxL}=28.125 \times 70=1968.75 \mathrm{~m}^{3}$.
b) Area of $\mathrm{c} / \mathrm{s}$ at one end $\mathrm{A}_{1}=\mathrm{bd}+\mathrm{nd}^{2}=8 \mathrm{x} 2+2 \times 2^{2}=24 \mathrm{~m}^{2}$

Area of $\mathrm{C} / \mathrm{s}$ at other end $\mathrm{A} 2=\mathrm{bd}+\mathrm{nd}^{2}=8 \times 2.5+2 \times 2.5^{2}=32.5 \mathrm{~m}^{2}$

Volume of earth work (V)=AmxL=28.25x70=1977.5m ${ }^{3}$.

## Example 7.3

The following width of road embank ment is 10 m . The side slopes are $2: 1$ The depth along the centre line road at 50 m intervals are $1.25,1.10,1.50,1.20$, $1.0,1.10,1.15 \mathrm{~m}$ calculate the Quantity of earth work by
a) Mid sectional rule
b) Trepezoidalrule
c) Prismoidalrule
a) Mid Sectional rule : $b=10 \mathrm{~m}, \mathrm{n}=2$.

| Chainage | Depths | Mean <br> epth $\left(\mathrm{d}_{\mathrm{m}}\right)$ | Area of <br> $\left(\mathrm{bd}_{\mathrm{m}}+\mathrm{nd}_{\mathrm{m}}^{2}\right)$ | Length b/w <br> Chainages | Quantity $\left(\mathrm{m}^{3}\right)$ <br> $\mathrm{A}_{\mathrm{m}} \times \mathrm{L}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 1.25 |  |  |  |  |
| 50 | 1.10 | 1.175 | 14.51 | 50 | 725.56 |
| 100 | 1.15 | 1.125 | 13.78 | 50 | 689.06 |
| 150 | 1.20 | 1.175 | 14.51 | 50 | 725.56 |
| 200 | 1.00 | 1.10 | 13.4 | 50 | 671.00 |
| 250 | 1.10 | 1.125 | 13.78 | 50 | 50 |
| 300 | 1.15 |  |  | 635.25 |  |

## b) Trepezoidal rule

$$
\begin{aligned}
& \mathrm{A}=\mathrm{bd}+\mathrm{nd}^{2} \\
& \mathrm{~A}_{1}=\mathrm{bd} 1+\mathrm{nd}_{1}^{2}=10 \times 1.25+2 \times 1.252=15.625 \mathrm{~m}^{2} \\
& \mathrm{~A}_{2}=\mathrm{bd} 2+\mathrm{nd}_{2}^{2}=10 \times 1.10+2 \times 1.10^{2}=13.42 \mathrm{~m}^{2} \\
& \mathrm{~A}_{3}=10 \times 1.15+2.1 .15^{2}=14.145 \mathrm{~m}^{2} \\
& \mathrm{~A}_{4}=10 \times 1.2+2 \times 1.2^{2}=14.88 \mathrm{~m}^{2} \\
& \mathrm{~A}_{5}=10 \times 1.0+2 \times 1^{2}=12.0 \mathrm{~m}^{2}, \\
& \mathrm{~A}_{6}=10 \times 1.1+2 \times 1.1^{2}=13.42 \mathrm{~m}^{2} \\
& \mathrm{~A}_{7}=10 \times 1.15+2 \times 1.152=14.145 \mathrm{~m}^{2}
\end{aligned}
$$

Volume of earth work by Trepezoidal rule

$$
\begin{aligned}
\mathrm{v} & \left.=\mathrm{L}\left[\left(\frac{\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}}{2}\right)^{2}+\left(\mathrm{A}_{2}+\mathrm{A}+\ldots . \mathrm{A}\right)_{\mathrm{n}-1}\right)^{\prime}\right\rfloor \\
& =50\left\lfloor\left(\frac{15.625+14.145}{2}\right)+(13.42+14.145+14.818+12.0+13.42)\right. \\
& =4137.50 \mathrm{~m}^{3}
\end{aligned}
$$

## Earth work Calculations

## c) By Prismoidalrule

$$
\begin{aligned}
\mathrm{v} & =\frac{\mathrm{L}}{3}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}\right)+4(\text { even Areas })+2(\text { Odd Areas })\right] \\
& =\frac{\mathrm{L}}{3}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{7}\right)+4\left(\mathrm{~A}_{2}+\mathrm{A}_{4}+\mathrm{A}_{6}\right)+2\left(\mathrm{~A}_{3}+\mathrm{A}_{5}\right)\right] \\
& =\frac{50}{3}[(15.625+14.145)+4(13.42+14.88+13.42)+2(14.145+12)] \\
& =4149 \mathrm{~m}^{3}
\end{aligned}
$$

Example 7.4:- Estimate the Quantity of earth work for a portion of road from the following data

| Chainage | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| RL | 7.50 | 7.70 | 7.50 | 7.25 | 6.85 | 6.95 | 6.70 | 6.45 | 6.30 | 5.95 |

The formation level at Chainage 0 is 8.0 and having falling gradient of 1 in 100. The top width is 12 m and side slopes $1 \frac{1}{2}$ horizontal to 1 vertical assuming the transverse direction is in level calculate the quantity of earth work Take 1 chain $=20 \mathrm{~m}$ by using trepezoidol \& Prismoidol formula.


Sol :-

| $\mathrm{b}=12 \mathrm{~m}$ |  | $\mathrm{n}=5$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Chainage | Distance | Reduced | Formation | Depth(d) of |  | Area of |  |
|  |  | level | Level | Embank- ment | Cuting | $\begin{gathered} \text { Embank- } \\ \text { ment } \\ \text { bd }^{2} \text { nd }^{2} \end{gathered}$ | Cutting |
| 0 | 0 | 7.50 | 8.0 | 0.50 |  | 6.375 |  |
| 1 | 20 | 7.70 | 7.8 | 0.10 |  | 1.275 |  |
| 2 | 40 | 7.50 | 7.6 | 0.10 |  | 1.215 |  |
| 3 | 60 | 7.25 | 7.4 | 0.15 |  | 1.839 |  |
| 4 | 80 | 6.85 | 7.2 | 0.35 |  | 4.38 |  |
| 5 | 100 | 6.95 | 7.0 | 0.05 |  | 0.63 |  |
| 6 | 120 | 6.70 | 6.8 | 0.10 |  | 1.215 |  |
| 7 | 140 | 6.45 | 6.6 | 015 |  | 1.837 |  |
| 8 | 160 | 6.30 | 6.4 | 0.10 |  | 1.215 |  |
| 9 | 180 | 5.95 | 6.2 | 0.25 |  | 3.09 |  |

$$
\begin{aligned}
& \text { Trepezoidal formula : } \\
& \left.\left.\qquad \mathrm{V}=\quad \mathrm{L}\left(\frac{\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}}{2}\right)+\left(\mathrm{A}_{2}+\mathrm{A}+\ldots .+\mathrm{A}_{3}\right)_{\mathrm{n}-1}\right)^{7}\right\rfloor \\
& =20\left[\left(\frac{6.375+3.09}{2}\right)+\left(1.215+1.215+1.837+4.38+0.63+1.215+1.837+1.215^{7}\right]\right. \\
& =365.53 \mathrm{~m}^{3}
\end{aligned}
$$

Prismoidalformula:

$$
\begin{aligned}
& \mathrm{V}=\frac{L}{3}\left[\left(\underset{1}{A}+A_{n}\right)+4(\text { even areas })+2(\text { Odd } \text { areas })\right] \\
& =\frac{\mathrm{L}}{3}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{10}\right)+4\left(\mathrm{~A}_{2}+\mathrm{A}_{4}+\mathrm{A}_{6}+\mathrm{A}_{8}\right)+2\left(\mathrm{~A}_{3}+\mathrm{A}_{5}+\mathrm{A}_{7}+\mathrm{A}_{9}\right)\right] \\
& =\frac{20}{3}[(6.375+3.09+4(1.215+1.837+0.63+1.837)+ \\
& \left.=317.27 \mathrm{~m}^{3} \quad 2(1.215+4.38+1.815+1.215)\right]
\end{aligned}
$$

## Earth work Calculations

Example 7.5:- The road has the following data

| Chainage | 0 | 20 | 40 | 60 | 80 | 100 | 120 |
| :--- | :--- | :---: | :---: | :--- | :--- | :--- | :--- |
| RL of <br> Ground | 20.6 | 21.0 | 21.5 | 22.1 | 22.7 | 22.9 | 23.0 |

The formation level at chainage zero is 22.0 and having a rising gradient of 1 in 100 the top width is 12.0 m and side slopes are $1 \frac{1}{2}: 1$ Assuming the transverse direction is in level. calculate the quantity of earth work by
a) Trepezoidal formula
b) Prismoldal formula

| Chainage Distance | Reduced <br> level | Formation <br> Level |  | Depth (d)of <br> Embark- <br> ment | Cut- <br> ting | Embark- <br> ment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 20.6 | 22.0 | 1.40 |  | 19.74 | Cutting |
| 20 | 21.0 | 22.2 | 1.20 |  | 16.56 |  |
| 40 | 21.5 | 22.4 | 0.90 |  | 12.01 |  |
| 60 | 22.1 | 22.6 | 0.50 |  | 6.375 |  |
| 80 | 22.7 | 22.8 | 0.10 |  | 1.215 |  |
| 100 | 22.9 | 23.0 | 0.10 |  | 1.215 |  |
| 120 | 23.0 | 23.2 | 0.20 |  | 2.460 |  |


a) Trepezoidal formula:

$$
\begin{aligned}
& \text { Vol of earth work in embankment } \\
& \left.\mathrm{V}=\mathrm{L}\left\lfloor\left(\frac{\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}}{2}\right)^{2}+\left(\mathrm{A}_{2}+\mathrm{A}+\ldots . . . .+\mathrm{A}_{3}\right)^{7}\right)^{7}\right\rfloor \\
& =20\left\lceil\left(\frac{19.74+2.46}{2}\right)+(16.56+12.01+6.375+1.215+1.215)\right\rceil \\
& =969.5 \mathrm{~m}^{3}
\end{aligned}
$$

## b) Prismoidal formula

$$
\begin{aligned}
V & =\frac{L}{3}\left[\left(A_{1}+A_{n}\right)+4(\text { even Areas })+2(\text { Odd Areas })\right] \\
& =\frac{20}{3}[(19.74+2.46)+4(16.56+6.325+1.2+5)+2(12.01+1.215)] \\
& =968.33 \mathrm{~m}^{3}
\end{aligned}
$$

## Earth work Calculations

Example 7.6:-From the above problem if the formation level at 0th chainage in 20 m . Calculate the volume of earth work by using the formulas?

| Chainage | Reduced <br> level | Formation <br> Level | Depth (d)of <br> Embank- <br> ment |  | Area ofting |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 |  |  | -- | 0.60 | - Embank- $_{\text {ment }}$Cutting <br> bd+nd |  |
| 20 | 21.00 | 20.20 | -- | 0.80 | -- | 10.56 |
| 40 | 21.50 | 20.40 | --- | 1.10 | --- | 15.015 |
| 60 | 22.10 | 20.60 | -- | 1.50 | -- | 21.375 |
| 80 | 22.70 | 20.80 | -- | 1.90 | -- | 28.215 |
| 100 | 22.90 | 21.00 | -- | 1.90 | -- | 28.215 |
| 120 | 23.00 | 21.20 | -- | 1.80 | -- | 26.460 |



## a) Trepezoidal formula:

$$
\begin{aligned}
& V=L\left[\left(\frac{\left.A_{1}+A_{n}\right)}{2}\right)+\left(A_{2}+A_{3}+\ldots \ldots . .+A^{n-1}\right)^{\prime}\right\rfloor \\
& =20\left[\left(\frac{7.74+26.46}{2}\right)+(10.56+15.015+21.375+28.215+28.215)\right] \\
& =2409.6 \mathrm{~m}^{3}
\end{aligned}
$$

b) Prismoidal formulae:

$$
\begin{aligned}
& \mathrm{V}=\frac{{ }_{2}^{L}}{3}\left[\left(\underset{1}{A}+A_{n}\right)+4(\text { even areas })+2(\text { Odd } \text { areas })\right] \\
& \\
& \\
& =\frac{\mathrm{L}}{3}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{7}\right)+4\left(\mathrm{~A}_{2}+\mathrm{A}_{4}+\mathrm{A}_{6}\right)+2\left(\mathrm{~A}_{3}+\mathrm{A}_{5}\right)\right] \\
& \\
& =\frac{20}{3}[(7.74+26.46)+4(10.56+21.375+28.215)+ \\
& \\
& \\
& \left.=2408.4 \mathrm{~m}^{3} \quad 2(15.015+28.215)\right]
\end{aligned}
$$

Example 7.7:-From the same above problem 7.6 if the gradient is in 100 falling calculate the quantity of earth work by using the formulas

| Chainage | Reduced <br> level | Formation <br> Level | Depth (d)of <br> Embank- <br> ment |  | Areat <br> ting |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 20.60 | 20.00 | -- | 0.60 | -Embank- <br> ment | Cutting |
| 20 | 21.00 | 19.8 | -- | 1.20 | -- | 7.74 |
| 40 | 21.50 | 19.6 | --- | 1.90 | --- | 28.215 |
| 60 | 22.10 | 19.4 | -- | 2.70 | -- | 43.335 |
| 80 | 22.70 | 19.20 | -- | 3.50 | -- | 60.375 |
| 100 | 22.90 | 19.0 | -- | 3.90 | -- | 69.615 |
| 120 | 23.00 | 18.80 | -- | 4.20 | -- | 76.86 |

Earth work Calculations

a) Trepezoidol formulae:

$$
\begin{aligned}
& \text { Vol.of earth work in cutting } \\
\mathrm{V}=\mathrm{L} & \left.\left\lceil\left(\frac{\left.\mathrm{~A}_{1}+\mathrm{A}_{\mathrm{n}}\right)}{2}\right)+\left(\mathrm{A}_{2}+\mathrm{A}+\ldots . . . .+\mathrm{A}\right)^{\mathrm{n}-1}\right)^{7}\right\rfloor \\
& =20\left[\left(\frac{7.74+76.86}{2}\right)+(16.56+28.215+43.335+60.375+69.615)\right] \\
= & 5208 \mathrm{~m}^{3}
\end{aligned}
$$

b) Prismoidal formulae:
$\mathrm{V}=\frac{\mathrm{L}}{3}[(\mathrm{~A} 1+\mathrm{An})+4($ even areas $)+2($ Odd areas $)]$

$$
\begin{aligned}
& =\frac{\mathrm{L}}{3}\left[\left(\mathrm{~A}_{1}+\mathrm{A}_{7}\right)+4\left(\mathrm{~A}_{2}+\mathrm{A}_{4}+\mathrm{A}_{6}\right)+2\left(\mathrm{~A}_{3}+\mathrm{A}_{5}\right)\right] \\
& =\frac{20}{3}[(7.74+76.86)+4(16.56+43.335+69.615)+ \\
& =5198.8 \mathrm{~m}^{3}
\end{aligned}
$$

Estimation and Costing
Example 7.8:- From the problem 7.5 if the gradient is 1 in 100 raising upto 40th chainage and 1 in 100 falling ragient from 40th Chainage to 120th chainage. Calculate the vol of earth work by using the formulas.

| Chainage <br> $(\mathrm{m})$ | R.L. | F.L. | Depth (d)of . |  | Area of . |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Embank- <br> ment | Cutting | Embank <br> ment <br> bd+nd | Cutting <br> bd + nd $^{2}$ |
| 0 | 20.6 | 22.0 | 1.40 |  | 19.74 |  |
| 20 | 21.0 | 22.20 | 1.20 |  | 16.56 |  |
| 40 | 21.5 | 22.40 | 0.90 |  | 12.01 |  |
| 60 | 22.1 | 22.20 | 0.10 |  | 1.215 |  |
| 62.5 |  |  | 0.00 | 0.00 | 0.000 | 0.000 |
| 80 | 22.7 | 22.00 |  | 0.70 |  | 9.135 |
| 100 | 22.9 | 21.80 |  | 1.10 |  | 15.015 |
| 120 | 23.0 | 21.60 |  | 1.40 |  | 19.74 |



From similer triangel properties

$$
\begin{aligned}
& \frac{\mathrm{x}}{0.1}=\frac{20-\mathrm{x}}{0.7} \\
& 0.7 \mathrm{x}=(20-\mathrm{x}) 0.1 \\
& 0.7 \mathrm{x}=2-0.1 \mathrm{x} \\
& 0.7 \mathrm{x}+0.1 \mathrm{x}=2 \\
& 0.8 \mathrm{x}=2 \\
& \mathrm{x}=\frac{2}{0.8}=\frac{20}{8}=2.5
\end{aligned}
$$



## Earth work Calculations

vol of earth work in embankment

| Chainage | 0 | 20 | 40 | 60 | 62.5 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Area | 19.74 | 16.56 | 12.01 | 1.215 | 0.00 |

here the intervals are not equal so we have to take the seperate volumes from oth chainage to 60th chainage and 60th chainage to 62.5 chainage

$$
\begin{aligned}
\mathrm{V} & =\operatorname{Vol}(0-60)+\operatorname{vol}(60-62.5) \\
& \left.=20\left[\left(\frac{19.74+1.215}{2}\right)+(16.56+12.01)\right\rceil+2.5^{\lceil 1.215+0.00\rceil}\right] \\
& =782.46 \mathrm{~m}^{3}
\end{aligned}
$$

By Prismoidal

$$
\begin{aligned}
\mathrm{V} & =\frac{20}{3}[(19.74+1.215)+4 \times 16.56+2 \times 12.01]+\frac{2.5}{3}[(1.215+0.00)] \\
& =742.44 \mathrm{~m}^{3}
\end{aligned}
$$

Vol of earth work in cutting

| Chainage | 62.5 | 80 | 100 | 120 |
| :--- | :---: | :---: | :---: | :---: |
| Area | 0.00 | 9.135 | 15.015 | 19.74 |

Volume (v) $=\operatorname{vol}(62.5-80)+\operatorname{Vol}(80-120)$
By Tripezoidal formula

By Prismoidal

$$
\begin{aligned}
v & =\frac{17.5}{3}[0.9+135]+\frac{20}{3}[(9.135+19.74)+4 \times 15.015] \\
& =646.18 \mathrm{~m}^{3}
\end{aligned}
$$

